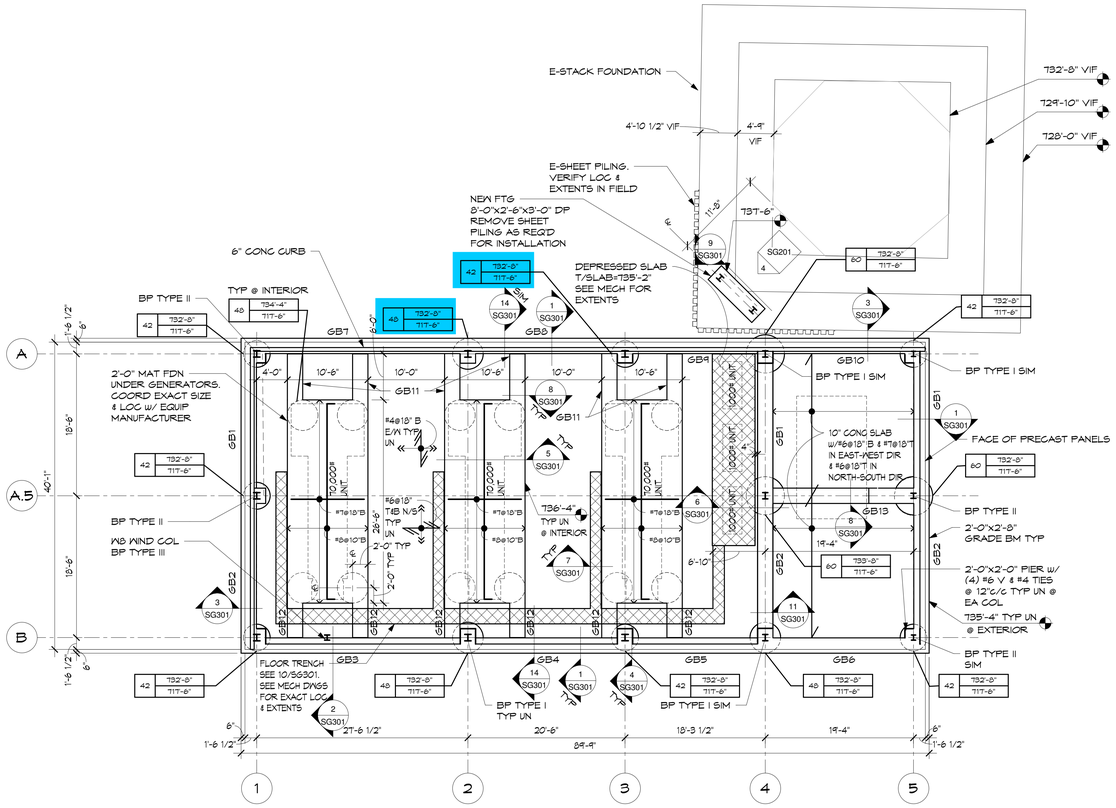
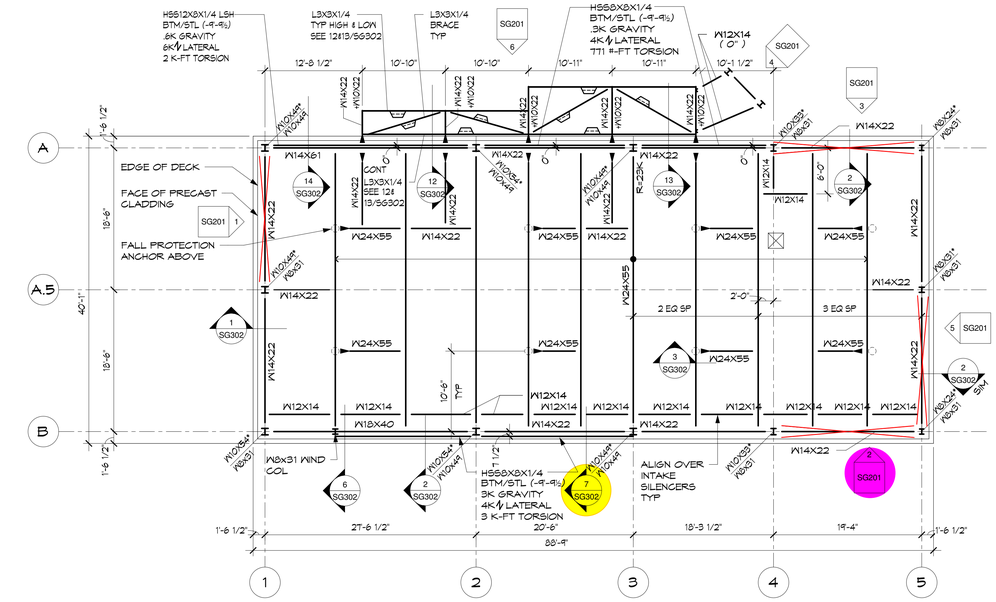


FOUNDATION PLAN

1/8" = 1'-0"

1. DESIGN LIVE LOAD: 100 PSF OR EQUIPMENT WEIGHT.
2. VERIFY LOCATIONS OF COLUMNS, WALLS, OPENINGS, MEP EQUIPMENT, ETC. WITH ARCHITECTURAL AND MEP DRAWINGS BEFORE PLACING FOUNDATIONS.
3. 6" STRUCTURAL SLAB ON 3" MUD MAT EXCEPT AS NOTED. SEE MEP FOR LOCATIONS AND QUANTITIES OF SLEEVES THROUGH SLAB.
4. TOP OF SLAB ELEVATION 736'-4" EXCEPT AS NOTED.
5. A= CAISSON DIAMETER IN INCHES.
   B= TOP OF CAISSON CUT OFF ELEVATION
   C= BOTTOM OF CAISSON BEARING ELEVATION. BOTTOM BEARING ELEVATION FOR ESTIMATING PURPOSES ONLY. BOTTOM OF CAISSON TO EXTEND TO SHALE OR CLAY SHALE MATERIAL MEETING DESIGN BEARING CAPACITY.
   DESIGN BEARING CAPACITY = 10KSF IN SHALE OR CLAY SHALE. THE DESIGN BEARING MATERIAL AND CAPACITY SHALL BE FIELD VERIFIED BY AN INDEPENDENT TESTING AGENCY SPECIALIZING IN SOILS INVESTIGATIONS.
6. INFORMATION FOR THE EXISTING BUILDING HAS BEEN TAKEN FROM DRAWINGS AND HAS NOT BEEN VERIFIED IN THE FIELD. CONTRACTOR SHALL VERIFY ALL RELEVANT CONDITIONS AND DIMENSIONS OF EXISTING CONSTRUCTION BEFORE PROCEEDING WITH THE WORK.
7. ELEVATIONS SHOWN ON PLAN ARE TOP OF THE FOOTING OR SLAB.
8. -INDICATES TOP OF FOOTING OR GRADE BEAM ON PLAN.
9. P -INDICATES SLEEVE FOR MEP. SEE MEP AND SITE UTILITY DRAWINGS FOR EXACT SIZE, ELEVATION & LOCATION.
10. ALL EXTERIOR FOOTINGS AND GRADE BEAMS TO EXTEND MINIMUM OF 2'-8" BELOW FINISHED GRADE.
11. DO NOT BACKFILL AGAINST THE FOUNDATION WALLS UNTIL BOTH LEVELS OF THE FLOOR SLAB ARE IN PLACE OR PROVIDE TEMPORARY SUPPORT. WHERE FILL IS ON BOTH SIDES OF A WALL, INSTALL THE FILL UNIFORMLY ON BOTH SIDES OF THE WALL.
12. REFERENCE: GENERAL STRUCTURAL NOTES - SG201, GRADE BEAM SCHEDULE - SG101, CAISSON SCHEDULE - SG101.

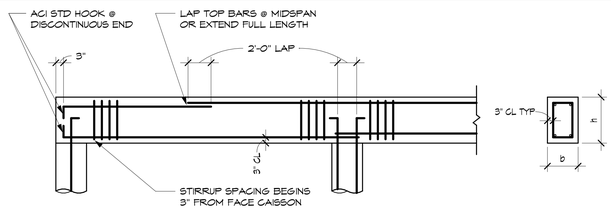


ROOF FRAMING PLAN

1/8" = 1'-0"

1. DESIGN LIVE LOADS: ROOF SNOW LOAD BASED ON 20 PSF GROUND SNOW LOAD.
2. TOP OF STEEL ELEVATION 762'-2" EXCEPT AS NOTED THUS (±).
3. [±] -INDICATES TOP OF STEEL ELEVATION RELATIVE TO THE SUPPORTING MEMBER.
4. ROOF CONSTRUCTION: 1½"X1B GAGE WIDE RIB GALVANIZED ACOUSTICAL METAL DECK.
5. 20K -INDICATES BEAM REACTION IN KIPS. SAME BOTH ENDS EXCEPT AS SHOWN. DESIGN CONNECTION FOR A MINIMUM OF 10 KIPS WHERE NO REACTION IS SHOWN.
6. -INDICATES A FRAMED OPENING ON PLAN. PROVIDE FRAME USING L5X3½X¼ LLV ON ALL SIDES. CONTRACTOR COORDINATE OPENING SIZES WITH MECHANICAL AND ARCHITECTURAL REQUIREMENTS. FRAMES ARE REQUIRED AT ROOF DRAINS. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR NUMBERS AND LOCATIONS.
7. -INDICATES MOMENT AND SHEAR CONNECTION. SEE SECTION 3/SG302.
8. * -INDICATES ALTERNATE COLUMN SIZE FROM FABRICATOR'S INVENTORY LIST. MAY BE USED AT FABRICATOR'S OPTION.
9. BEAMS ARE UNIFORMLY SPACED BETWEEN COLUMNS OR INTERSECTING GIRDERS UNLESS NOTED OTHERWISE.
10. REFERENCES: GENERAL STRUCTURAL NOTES - SG201.

GRADE BEAM SCHEDULE



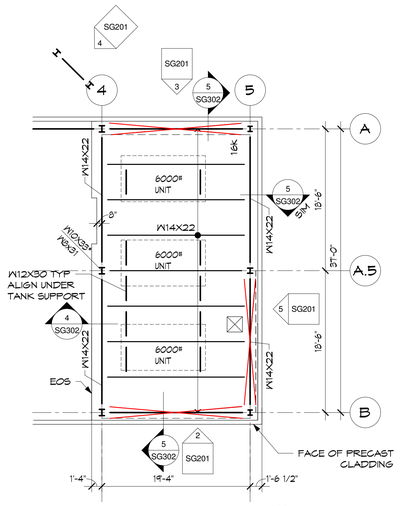
CONT - CONTINUOUS
FAB - FROM ADJACENT BEAM
EE - EACH END
FL - FULL LENGTH TO CUTOFF PT OF ADJACENT BEAM
DE - DISCONTINUOUS END
CE - CONTINUOUS END

MARK	b x h	REINFORCING BOTTOM	REINFORCING TOP	REINFORCING LOCATION	CLOSED STIRRUPS SIZE	CLOSED STIRRUPS SPACING	REMARKS
GB1	2'-0" x 2'-8"	(3) #9	(2) #5	DE	#4	3 @ 14", REMAINDER @ 24"	EXTEND TOP BARS INTO PIER @ DE.
			(3) #9	CE	#4	5 @ 14", REMAINDER @ 24"	
GB2	2'-0" x 2'-8"	(3) #9	(2) #5	DE	#4	3 @ 14", REMAINDER @ 24"	EXTEND TOP BARS INTO PIER @ DE.
			FAB	CE	#4	5 @ 14", REMAINDER @ 24"	
GB3	2'-0" x 2'-8"	(3) #9	(2) #5	1	#4	3 @ 14", REMAINDER @ 24"	
			(3) #9	2	#4	3 @ 14", REMAINDER @ 24"	
GB4	2'-0" x 2'-8"	(3) #9	FAB	2	#4	3 @ 14", REMAINDER @ 24"	
			(3) #9	3	#4	@ 24"	
GB5	2'-0" x 2'-8"	(3) #9	FAB	3	#4	@ 24"	
			(3) #9	4		3 @ 14", REMAINDER @ 24"	
GB6	2'-0" x 2'-8"	(3) #9	FAB	4	#4	5 @ 14", REMAINDER @ 24"	
			(2) #5	5	#4	1 @ 14", REMAINDER @ 24"	
GB7	2'-0" x 2'-8"	(3) #9	(2) #5	1	#4	5 @ 14", REMAINDER @ 24"	
			(3) #9	2	#4	5 @ 14", REMAINDER @ 24"	
GB8	2'-0" x 2'-8"	(3) #9	FAB	2	#4	5 @ 14", REMAINDER @ 24"	
			(3) #9	3	#4	@ 24"	
GB9	2'-0" x 2'-8"	(3) #9	FAB	3	#4	@ 24"	
			(3) #9	4	#4	5 @ 14", REMAINDER @ 24"	
GB10	2'-0" x 2'-8"	(3) #9	FAB	4	#4	5 @ 14", REMAINDER @ 24"	
			(2) #5	5	#4	@ 24"	
GB11	2'-0" x 2'-0"	(2) #5	(2) #5	CONT	#4	@ 24"	EXTEND T&B BARS 3'-0" INTO GENERATOR PAD.
GB12	2'-0" x 2'-0"	(3) #4	(2) #5	CONT	#4	@ 8"	EXTEND T&B BARS 3'-0" INTO GENERATOR PAD.
GB13	2'-0" x 2'-8"	(3) #9	(2) #5	EE	#4	5 @ 14", REMAINDER @ 24"	EXTEND TOP BARS INTO PIER @ DE.

CAISSON SCHEDULE

DIAMETER	REINFORCING
3'-6"	(7) #9V, #4 TIES @ 18" C/C
4'-0"	(7) #10V, #4 TIES @ 18" C/C
5'-0"	(9) #11V, #4 TIES @ 18" C/C

NOTE:
1) CONTRACTOR MAY UTILIZE LARGER DIAMETER CAISSONS FOR SIZES SHOWN. SUBMIT PROPOSED SIZES FOR REVIEW.
2) PROVIDE (4) #6 DWLS FROM CAISSON TO GRADE BEAM WHERE NO COLUMN PIER W/VERT BARS IS SHOWN.
3) VERTICAL REINFORCING NEED ONLY EXTEND 3 TIMES THE CAISSON DIAMETER OR 10'-0", WHICHEVER IS GREATER, INTO THE CAISSON.



MEZZANINE FRAMING PLAN

1/8" = 1'-0"

1. DESIGN LIVE LOAD: 100 PSF.
2. TOP OF STEEL ELEVATION 751'-0" EXCEPT AS NOTED THUS (±).
3. FLOOR CONSTRUCTION: 4" CONCRETE ON 1½"X22 GAGE NON-COMPOSITE METAL DECK w/6x6-W2.9xW2.9 WWR.
4. 20K -INDICATES BEAM REACTION IN KIPS. SAME BOTH ENDS EXCEPT AS SHOWN. DESIGN CONNECTION FOR A MINIMUM OF 10 KIPS WHERE NO REACTION IS SHOWN.
5. 20K ↯ -INDICATES BEAM UNFACTORED LATERAL REACTION AND DIRECTION IN KIPS.
6. -INDICATES A FRAMED OPENING ON PLAN. PROVIDE FRAME USING L5X3½X¼ LLV ON ALL SIDES. CONTRACTOR COORDINATE OPENING SIZES WITH MECHANICAL AND ARCHITECTURAL REQUIREMENTS. FRAMES ARE REQUIRED AT ROOF DRAINS. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR NUMBERS AND LOCATIONS.
7. * -INDICATES ALTERNATE COLUMN SIZE FROM FABRICATOR'S INVENTORY LIST. MAY BE USED AT FABRICATOR'S OPTION..
8. BEAMS ARE UNIFORMLY SPACED BETWEEN COLUMNS OR INTERSECTING GIRDERS UNLESS NOTED OTHERWISE.
9. REFERENCES: GENERAL STRUCTURAL NOTES - SG201.

ENTIRE SHEET REISSUED 1

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NO.	DATE	DESCRIPTION
18	4-6-11	Bulletin 018 - Generator Building Revision
9	12-22-10	Bid Package 3 - Addendum 1
8	12-14-10	Bid Documents - Bid Package 3 - Bid Issuance
7	11-30-10	Bid Documents - Bid Package 3 - CD Back Check Set
6	11-17-10	Bid Documents - Bid Package 3 - Revised CD Review Set
5	10-15-10	Bid Documents - Bid Package 3 - CD Review Set
4	09-17-10	Bid Documents – Bid Pack 2 – Back Check
3	07-26-10	ISSUED FOR PERMIT (FOUNDATION ONLY)
2	07-22-10	CD - BID PACK 2
1	05-07-10	DD - ADDENDUM

REVISIONS

NOT FOR CONSTRUCTION
- CONCEPTUAL DESIGN
- SCHEMATICS
- DESIGN DEVELOPMENT
- CONSTRUCTION DOCUMENTS

THIS DOCUMENT WAS ORIGINALLY ISSUED BY STEPHEN J METZ ON 7-22-10. THIS DOCUMENT IS NOT CONSIDERED A SEALED DOCUMENT.

OSU PROJECT NUMBER OSU-090344

SOUTH CAMPUS CENTRAL CHILLER PLANT

PREPARED FOR
THE OHIO STATE UNIVERSITY
FACILITIES OPERATIONS AND DEVELOPMENT
COLUMBUS, OHIO

PREPARED BY
CHAMPLIN ARCHITECTURE
424 EAST FOURTH STREET *CINCINNATI, OHIO 45202*

FOUNDATION & ROOF PLAN

SHEET **SG101**

O.S.U. PROJ. NO.	FILENAME	DRAWN BY	DATE	377 X XXXX
OSU-090344	J:\2504055	SMBH	4-6-11	

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GENERAL STRUCTURAL NOTES

1. THE GENERAL STRUCTURAL NOTES ARE INTENDED TO AUGMENT THE DRAWINGS AND SPECIFICATIONS. SHOULD CONFLICTS EXIST BETWEEN THE DRAWINGS AND SPECIFICATIONS AND THE GENERAL STRUCTURAL NOTES, THE STRICTEST PROVISION SHALL GOVERN.
2. GOVERNING CODE: OHIO BUILDING CODE - 2007 EDITION.
3. SEE STRUCTURAL PLANS FOR DESIGN SOIL BEARING PRESSURE AND LIVE LOADS. LIVE LOADS REDUCED IN ACCORDANCE WITH THE GOVERNING CODE.
4. ROOF SNOW LOAD:

GROUND SNOW LOAD (Pg)	- 20 PSF
SNOW EXPOSURE FACTOR (Ce)	- 1.0
IMPORTANCE FACTOR (Is)	- 1.2
THERMAL FACTOR (Ct)	- 1.0
FLAT ROOF SNOW LOAD (Pf)	- 24 PSF

5. WIND LOAD:

BASIC WIND SPEED	- 90 MPH
IMPORTANCE FACTOR (Iw)	- 1.15
EXPOSURE CATEGORY	- EXPOSURE C
INTERNAL PRESSURE COEFFICIENT - (G Cpi)	- ±0.55

6. SEISMIC LOAD:

OCCUPANCY CATEGORY	- IV
IMPORTANCE FACTOR (Ie)	- 1.5
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (Ss)	- 0.15
MAPPED SPECTRAL RESPONSE ACCELERATION AT ONE-SECOND PERIOD(S1)	- 0.059
SITE CLASS	- C
SPECTRAL RESPONSE COEFFICIENT AT SHORT PERIOD (SDs)	- 0.12
SPECTRAL RESPONSE COEFFICIENT AT ONE-SECOND PERIOD (SD1)	- 0.067
SEISMIC DESIGN CATEGORY	- C
DESIGN BASE SHEAR	- 11 K
SEISMIC RESPONSE COEFFICIENT (Cs)	- 0.038

   BASIC SEISMIC FORCE RESISTING SYSTEM:
   H-STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE (R=3, Ω=3)
   DESIGN BY EQUIVALENT LATERAL FORCE PROCEDURE.
7. MECHANICAL FRAMING LOADS, OPENINGS, AND STRUCTURE IN ANY WAY RELATED TO MECHANICAL REQUIREMENTS ARE SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL OBTAIN APPROVAL OF MECHANICAL AND OTHER TRADES BEFORE PROCEEDING WITH SUCH PORTION OF THE WORK. EXCESS COST RELATED TO VARIATION IN MECHANICAL REQUIREMENTS TO BE BORNE BY MECHANICAL CONTRACTOR. COORDINATE SIZE AND LOCATION OF ALL OPENINGS WITH THE MECHANICAL DRAWINGS.
8. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCES AND TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS, OR TIE-DOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER COMPLETION OF THE PROJECT.
9. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
10. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS RELATING TO EXISTING CONSTRUCTION AND EXISTING SERVICE ON THE SITE.
11. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF COLUMNS, WALLS, OPENINGS ETC. WITH THE ARCHITECTURAL DRAWINGS PRIOR TO PROCEEDING WITH THE WORK.

REINFORCED CONCRETE

1. SPECIFICATIONS AND STANDARDS:
   UNLESS SPECIFICALLY SHOWN OTHERWISE, ALL CONCRETE WORK, DETAILING, FABRICATION AND PLACING OF BARS AND CONCRETE SHALL BE GOVERNED BY:
   A. ACI 117-06, ACI 301-05, ACI 315-99, AND ACI 318-05.
   B. CRSI RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS.
   C. ACI 308-92 AND ACI 305-06 FOR WINTER AND HOT WEATHER CONCRETING, RESPECTIVELY. THE CONTRACTOR SHALL AT ALL TIMES HAVE A COPY OF THE RELEVANT SPECIFICATIONS QUOTED ABOVE ON THE SITE AND THE SUPERVISORY PERSONNEL SHALL BE THOROUGHLY FAMILIAR WITH THE CONTENTS THEREOF.
2. CONTINGENCIES:
   A. PROVIDE LEAN CONCRETE UNDER FOUNDATIONS FOR EARTH FILL DUE TO ACCIDENTAL OVER EXCAVATION OR SOFT SPOTS.
3. CONCRETE REQUIREMENTS AND LOCATION IN JOB:

CLASS	LOCATION	F'C	SPECIAL REQUIREMENTS
1	CAISSONS AND ALL INTERIOR CONCRETE NOT OTHERWISE NOTED.	3000 PSI	
2	EXTERIOR CONCRETE	4000 PSI	5% ±1% AIR CONTENT
3	INTERIOR SLABS, GRADE BEAMS, & SLABS ON METAL DECK	3500 PSI	
5	EARTH FILL & MUDMAT	1500 PSI	NO TESTS

   SUBMIT CONCRETE MIXES FOR APPROVAL IN ACCORDANCE WITH ACI 301 BEFORE PLACING ANY CONCRETE.
4. REINFORCING REQUIREMENTS:
   A. BARS: ASTM A615, A616, A617 - GRADE 60.
   B. WELDED WIRE REINFORCING: ASTM A185.
5. FOOTINGS:
   A. DOWELS IN FOOTINGS TO MATCH VERTICAL REINFORCING IN CONCRETE WALLS, COLUMNS OR PIERS. DOWELS IN FOOTING TO MASONRY WALLS NOT REQUIRED UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DETAILS.
   B. BEND ALL BARS 24 DIAMETERS AROUND CORNERS OF FOOTINGS. BARS AT THE INSIDE FACE OF THE CORNER SHALL BE CONTINUED ACROSS TO THE OUTSIDE AND THEN BENT.
6. MISCELLANEOUS:
   A. IF NO OTHER REINFORCING IS SHOWN IN A SLAB ON GRADE, PROVIDE 6x6-W1.4x W1.4 WWR AT MID-THICKNESS OF SLAB.
   B. LAP WELDED WIRE REINFORCING 1 SPACE + 2" AT ALL EDGES AND ENDS OF SHEET.
   C. DO NOT BACKFILL AGAINST FOUNDATION WALLS UNTIL BOTH ADJACENT FLOOR SLABS ARE IN PLACE OR WHERE FILL IS ON BOTH SIDES OF A WALL, BRING THE FILL UP UNIFORMLY.
   D. CONSTRUCTION JOINTS PERMITTED ONLY WHERE SHOWN OR AS APPROVED BY THE STRUCTURAL ENGINEER.
   E. PROVIDE ONE #4x3'-0" DIAGONAL REINFORCING BAR AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNERS OF SLABS ON GRADE OR SLABS ON METAL DECK.
   F. PROVIDE SLAB BOLSTERS TO CORRECTLY LOCATE WELDED WIRE REINFORCING AT MID-DEPTH OF CONCRETE SLABS ON GRADE.

STRUCTURAL STEEL

1. SPECIFICATIONS AND STANDARDS:
   UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
   A. AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, ASD-13th EDITION
   B. AISC CODE OF STANDARD PRACTICE - 2005.
   C. AWS STANDARD WELDING SYMBOLS.
   D. STRUCTURAL WELDING CODE AWS D1.1-2006 OF THE AMERICAN WELDING SOCIETY. WELDING SHALL BE PERFORMED ONLY BY OPERATORS QUALIFIED, BY THE AWS STANDARD QUALIFICATION PROCEDURE, TO PERFORM THE PARTICULAR TYPE OF WORK REQUIRED.
   E. SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS - 2004.
2. TESTING:
   A. WELDS: VISUAL TESTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY ON ALL CRITICAL WELDS AND ON 25% OF NONCRITICAL WELDS. INADEQUATE WELDS SHALL BE STRENGTHENED OR CUT OUT AND REPLACED AS DIRECTED. CRITICAL WELDS SHALL BE DEFINED AS ALL FULL PENETRATION WELDS, ALL WELDS IN MOMENT CONNECTIONS AND AS NOTED AS CRITICAL WELDS ON THE STRUCTURAL DETAILS.
   B. STRUCTURAL STEEL: PROVIDE MILL REPORTS FOR PROPERLY IDENTIFIED MATERIALS ON REQUEST.
   C. A325 AND A490 BOLTS: PROVIDE BOLT INSPECTION AS DETAILED IN PARAGRAPH 9 OF SPECIFICATIONS FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS.
3. MATERIALS:
   A. "W" SHAPES: ASTM A992 Fy = 50 KSI
   B. CHANNELS: ASTM A36
   C. ANGLES, PLATES AND BARS: ASTM A36.
   D. WELDING ELECTRODES: AWS A5.1 OR A5.5 SERIES E70.
   E. BOLTS: ASTM A325.
   F. ANCHOR BOLTS: ASTM F1554 GR.36
   G. PAINT AND PROTECTION:
      1. PRIME COAT, TOUCH UP AFTER ERECTION.
      2. MEMBERS TO BE ENCASED IN CONCRETE OR SPRAY-ON FIRE PROOFING: NO PAINT.
      3. MEMBERS EXPOSED TO WEATHER IN FINISHED STRUCTURE: GALVANIZED AFTER FABRICATION.
4. CONNECTION REQUIREMENTS:
   A. DESIGN CONNECTIONS FOR VERTICAL REACTIONS SHOWN ON DRAWINGS OR FOR FULL CAPACITY OF MEMBER WHERE NO REACTION IS SHOWN.
   B. DESIGN MOMENT BEAM CONNECTIONS FOR VALUES SHOWN OR FOR FULL MOMENT CAPACITY OF MEMBER.
   C. CONNECTIONS SHOWN AND DETAILED ON THE DRAWINGS MAY BE REDESIGNED BY THE STRUCTURAL STEEL CONTRACTOR FOR EQUAL FORCES PROVIDED THE SAME ARRANGEMENT OF MEMBERS IS USED AND THE OVERALL SIZE OF THE CONNECTION DOES NOT EXCEED THAT OF THE CONNECTION DETAILED.
   D. OBTAIN APPROVAL FROM STRUCTURAL ENGINEER FOR TYPES OF CONNECTIONS BEFORE FABRICATION.
   E. ALL BOLTED CONNECTIONS TO BE SHEAR/BEARING TYPE WITH BOLTS IN THE SNUG TIGHT CONDITION UNLESS NOTED OTHERWISE.
5. MISCELLANEOUS REQUIREMENTS:
   A. PROVIDE HOLES FOR OTHERS. IF SECTION IS WEAKENED BY MORE THAN 15% BY AN OPENING NOT SHOWN ON THE DRAWINGS, OBTAIN PRIOR APPROVAL.
   B. STEEL SUPPORTING OR CONNECTING TO MECHANICAL OR OTHER EQUIPMENT IS SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL RECONCILE EXACT SIZE AND LOCATION WITH MECHANICAL AND OTHER REQUIREMENTS BEFORE PROCEEDING WITH THE WORK. CONTRACTOR SHALL COORDINATE EXACT SIZE AND LOCATION FOR ALL STEEL ANGLE FRAMES WITH OPENINGS SHOWN ON THE MECHANICAL AND ARCHITECTURAL DRAWINGS.
   C. GROUT UNDER BEARING PLATES TO BE NON-SHRINKING TYPE MEETING ALL THE REQUIREMENTS OF CRD-C621, CORPS OF ENGINEERS SPECIFICATION FOR NON-SHRINK GROUT OR MEETING ALL REQUIREMENTS OF ASTM C1107 FOR FLUID CONSISTENCY, 30 MINUTE WORKING TIME AND TEMPERATURE RANGE FROM 45°F TO 90°F.
   D. STEEL BELOW GRADE TO BE PROTECTED BY A MINIMUM OF 3" OF CONCRETE OR 4" OF MASONRY.
   E. PROVIDE HEAVY WASHER AT ALL ANCHOR BOLTS.
   F. PROVIDE ANGLE SUPPORTS FOR ALL METAL DECK RIBS AT COLUMNS WHEN COLUMN SIZE PREVENTS RIB FROM CONTINUING TO BEAM WHICH IS SUPPORTING DECK AT COLUMN LINE.
6. DRILLED ANCHORS:
   A. PROVIDE DRILLED ANCHORS AS INDICATED ON THE STRUCTURAL DRAWINGS. DRILLED EXPANSION ANCHORS SHALL BE WEDGE TYPE WITH ONE PIECE WRAP AROUND EXPANSION CLIP. THE ENTIRE ANCHOR SHALL BE CARBON STEEL MEETING THE FOLLOWING REQUIREMENTS, AND SHALL BE EVALUATED TO COMPLY WITH IBC 2006 INCLUDING, BUT NOT LIMITED TO, SECTION 1912 AND ACI 318 APPENDIX D SECTION D.3.3.

ANCHOR SIZE	MINIMUM EMBEDMENT	PULLOUT (LBS)	SHEAR (LBS)
¼" DIAMETER	2"	550	590
⅜" DIAMETER	2½"	1050	1300
½" DIAMETER	3½"	1700	2200
⅝" DIAMETER	4"	2400	3300
¾" DIAMETER	4½"	3250	4650
1" DIAMETER	6"	6400	7450

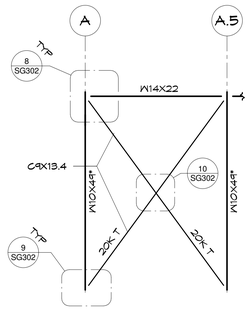
   LOADS INDICATED ABOVE ARE SERVICE LOADS IN POUNDS FOR STAINLESS STEEL ANCHORS IN 3000 PSI STONE AGGREGATE CONCRETE.
7. EPOXY ANCHORS:
   A. PROVIDE DRILLED ANCHORS EMBEDDED IN ADHESIVE AS INDICATED ON THE STRUCTURAL DRAWINGS. THE ENTIRE ANCHOR SYSTEM SHALL BE EVALUATED TO COMPLY WITH IBC 2006 INCLUDING, BUT NOT LIMITED TO, SECTION 1912 AND ACI 318 APPENDIX D SECTION D.3.3.

ANCHOR SIZE	MINIMUM EMBEDMENT	PULLOUT (LBS)	SHEAR (LBS)
⅜" DIAMETER	3½"	1800	1050
½" DIAMETER	4½"	2700	1900
⅝" DIAMETER	5"	4300	3000
¾" DIAMETER	7"	4650	4350
⅞" DIAMETER	7"	7800	5900
1" DIAMETER	8¼"	14500	9600

   LOADS INDICATED ABOVE ARE SERVICE LOADS FOR ANCHORS IN 3000 PSI STONE AGGREGATE CONCRETE.

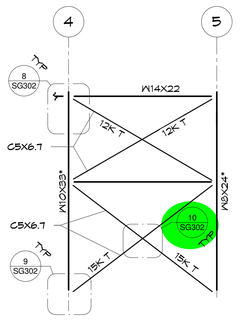
STEEL DECK

1. SPECIFICATIONS AND STANDARDS:
   A. UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN FABRICATION AND ERECTION OF STEEL DECK SHALL BE GOVERNED BY THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE, SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
   B. ALL PROPERTIES OF THE STRUCTURAL STEEL DECK SHALL BE COMPUTED IN ACCORDANCE WITH THE REFERENCE STANDARD. THE PROPERTIES SHALL BE PUBLISHED IN THE MANUFACTURER'S CATALOGS.
   C. AWS STANDARD WELDING SYMBOLS.
   D. AWS D1.3 SPECIFICATIONS FOR WELDING SHEET STEEL IN STRUCTURES.
   E. WELDING SHALL BE PERFORMED ONLY BY OPERATORS QUALIFIED, BY THE AWS STANDARD QUALIFICATION PROCEDURE, TO PERFORM THE PARTICULAR TYPE OF WORK REQUIRED.
2. MATERIALS:
   A. GALVANIZED STEEL DECK: ASTM A653 STRUCTURAL QUALITY GRADE 33 WITH COATING DESIGNATION G60.
   B. WELDING ELECTRODES: AWS A5.1, A5.5 OR A5.18 SERIES E60.
3. ERECTION AND CONNECTIONS:
   A. STEEL DECK UNITS AND ACCESSORIES SHALL BE AS SHOWN ON THE MANUFACTURER'S ERECTION DRAWINGS.
   B. MINIMUM BEARING OF THE DECK SHALL BE 2 INCHES UNLESS OTHERWISE SHOWN.
   C. ANCHOR STEEL DECK TO STEEL SUPPORTING MEMBERS WITH ⅝" PUDDLE WELDS AT A MAXIMUM AVERAGE SPACING OF 12 INCHES UNLESS SHOWN OTHERWISE.
   D. SIDELAPS SHALL BE FASTENED BY MEANS OF A #10 SCREW AT MIDSPAN BETWEEN SUPPORTS UNLESS SHOWN OTHERWISE.
4. OPENINGS IN STEEL DECK:
   A. OPENINGS CUT IN THE STEEL DECK SHALL BE REINFORCED OR SHALL BE SUPPORTED ON STEEL ANGLE FRAMES. COORDINATE SIZES AND LOCATIONS WITH THE MECHANICAL AND ARCHITECTURAL DRAWINGS.
   B. OPENINGS IN STEEL DECK EQUAL TO OR LESS THAN 12"x12" SHALL BE REINFORCED WITH A 24"x24" - 16 GAGE PLATE SCREWED OR WELDED TO THE DECK RIBS ON ALL SIDES OF THE OPENING.
   C. OPENINGS IN ROOF DECK GREATER THAN 12"x12" SHALL BE SUPPORTED ON STEEL ANGLE FRAMES.



1 SG201 ELEVATION 1/8" = 1'-0"

1. * INDICATES COLUMN SIZE FROM FABRICATOR'S INVENTORY LIST. ALTERNATELY, ALL COLUMN SIZES MAY BE W8x31.



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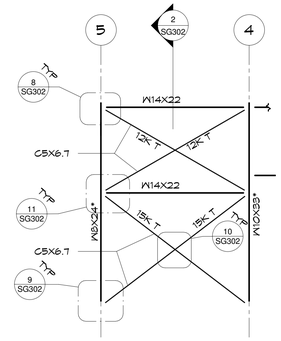

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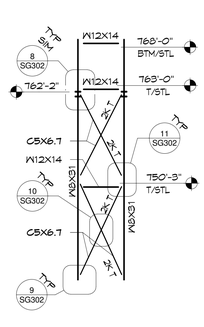
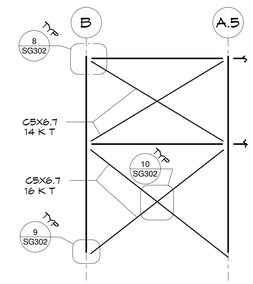


4 SG201 ELEVATION 1/8" = 1'-0"

1. GALVANIZE ALL MEMBERS EXPOSED TO WEATHER IN FINISHED STRUCTURE, SEE STRUCTURAL STEEL NOTES, SHEET SG201.



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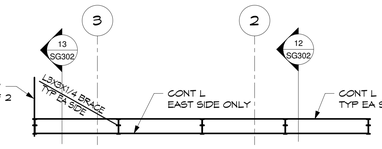
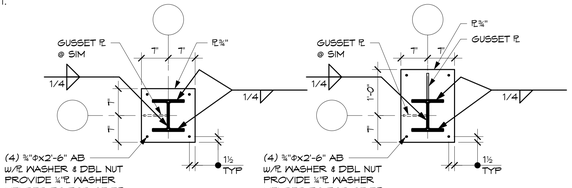


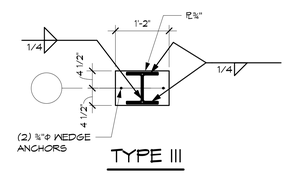
6 SG201 EXHAUST SUPPORT ELEVATION 1/8" = 1'-0"

1. GALVANIZE ALL MEMBERS EXPOSED TO WEATHER IN FINISHED STRUCTURE, SEE STRUCTURAL STEEL NOTES, SHEET SG201.

ABBREVIATIONS

AB	Anchor Bolt	JT	Joint
ADD'L	Additional	K	Kip (1000 pounds)
ADJ	Adjacent	KSF	Kips per Square Foot
AFF	Above Finished Floor	LAT	Lateral
ANC	Anchor	LBS, #	Pounds
APPROX	Approximately	LL	Live Load
ARCH	Architect (ural)	LLBB	Long Leg Back to Back
B, BOT	Bottom	LLH	Long Leg Horizontal
BLDG	Building	LLV	Long Leg Vertical
BM	Beam	LNTL	Lintel
BS	Both Sides	LOC	Location
B P.	Base Plate	LSH	Long Side Horizontal
BTWN	Between	LSL	Long Slotted Holes
C/C	Center-to-Center	LSV	Long Side Vertical
CJ	Control Joint	LW	Long Way
CL	Clear	MAS	Masonry
CMU	Concrete Masonry Units	MAX	Maximum
COL	Column	MECH	Mechanical
CONC	Concrete	MEZZ	Mezzanine
CONN	Connect (ion)	MIN	Minimum
CONST	Construct (ion)	MPH	Miles per Hour
CONT	Continuous (ation)	MISC	Miscellaneous
CONTR	Contractor	MTL	Metal
CTR	Center	N/A	Not Applicable
CW	Curtain Wall	NIC	Not in Contract
DBL	Double	NS	Near Side
DE	Discontinuous End	NTS	Not to Scale
DET	Detail	OPNG	Opening
DIA Φ	Diameter	OPP	Opposite (Hand)
DIAG	Diagonal	OC	On Center
DIM	Dimension	OD	Outside Diameter
DL	Dead Load	OF	Outside Face
DWG	Drawing	O/O	Out-to-Out
DWL	Dowel	OSL	Outstanding Leg
EA	Each	OVS	Oversize Round Holes
EE	Each End	P/C	Precast Concrete
EF	Each Face	PAF	Powder Actuated Fastener(s)
EJ	Expansion Joint	PLF	Pounds per Linear Foot
EL	Elevation	PSI	Pounds per Square Inch
ELEV	Elevator	PSF	Pounds per Square Foot
EMBED	Embedded (ment)	QTY	Quantity
ENGR	Engineer	RD	Roof Drain
EOS	Edge of Slab	REINF	Reinforce (ing) (ed)
EQ	Equal	REQ	Require (ments)
E-, EXTG	Existing	REQD	Required
ES	Each Side	SC	Slip-critical
EW	Each Way	SCHED	Schedule
EXP	Expansion	SHT	Sheet
EXT	Exterior	SIM	Similar
FAB	From Adjacent Beam	SLBB	Short Leg Back to Back
FABR	Fabricate (or)	SPEC	Specification(s)
FFE	Finished Floor Elevation	SL	Slope(d)
FIN	Finished	SP	Space(s) (ed)
FL	Full Length	SQ	Square
FLR	Floor	SSL	Short Slotted Holes
FND	Foundation	STIFF	Stiffener
FOM	Face of Masonry	STD	Standard
FOS	Face of Stud	STL	Steel
FOV	Face of Veneer	STRUCT	Structure (al)
FOW	Face of Wall	SW	Short Way
FOB	Face of Building	T	Top
FS	Far Side	T/	Top of
FT	Feet, Foot	TEMP	Temperature, Temporary
FTG	Footing	THD	Threaded
GA	Gage	THK	Thick (ness)
GALV	Galvanized	T&B	Top and Bottom
GC	General Contractor	TOS	Top of Steel
GR	Grade	TYP	Typical
GRTG	Grating	UN	Unless Noted
GEN	General	UNO	Unless Noted Otherwise
H, HORIZ	Horizontal	V, VERT	Vertical
HD	Headed	W/	With
HK	Hook	WWR	Welded Wire Reinforcing
ID	Inside Diameter	WP	Work(ing) Point
IF	Inside Face	XS	Extra Strong
INT	Interior, Intermediate	XXS	Double Extra Strong
JBE	Joist Bearing Elevation	℄	Centerline
		ℙ	Plate
		#	Number





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MARTIN CONTROLS SYSTEMS INC., 5955 Willow Place, Dublin, Ohio 43016, Tel. (614) 761-5600, Fax. (614) 761 5601

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OSU PROJECT NUMBER OSU-090344

SOUTH CAMPUS CENTRAL CHILLER PLANT

PREPARED FOR
THE OHIO STATE UNIVERSITY
FACILITIES OPERATIONS AND DEVELOPMENT
COLUMBUS, OHIO

PREPARED BY
CHAMPLIN ARCHITECTURE
424 EAST FOURTH STREET CINCINNATI, OHIO 45202

GENERAL NOTES AND ELEVATIONS

SHEET SG201

O.S.U. PROJ. NO.	FILENAME	DRAWN BY	DATE	
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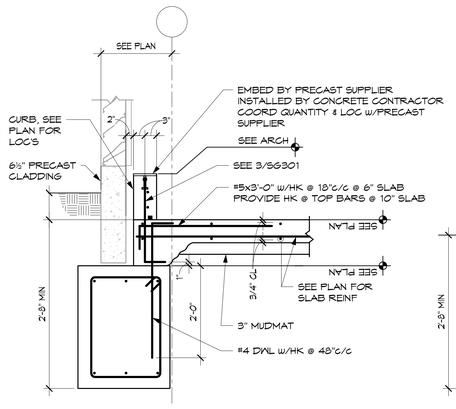

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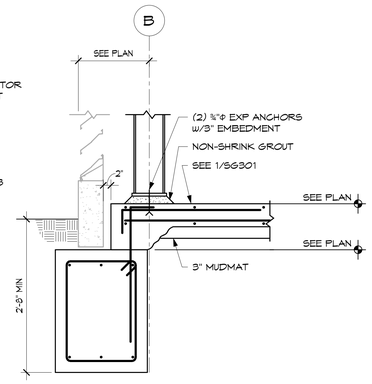

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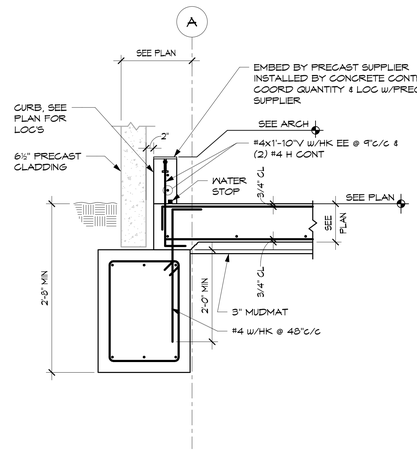

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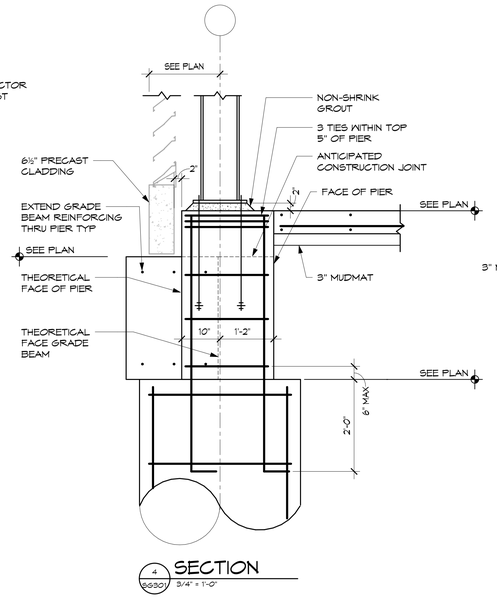

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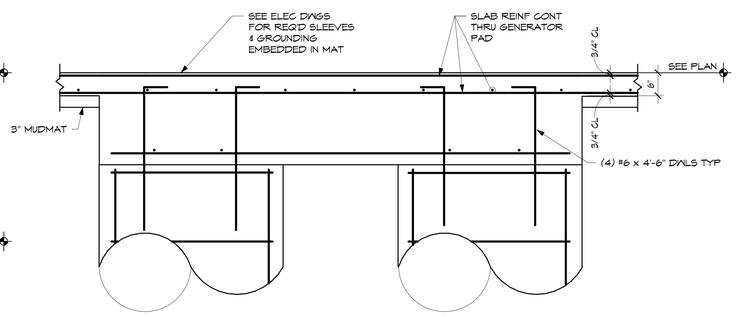

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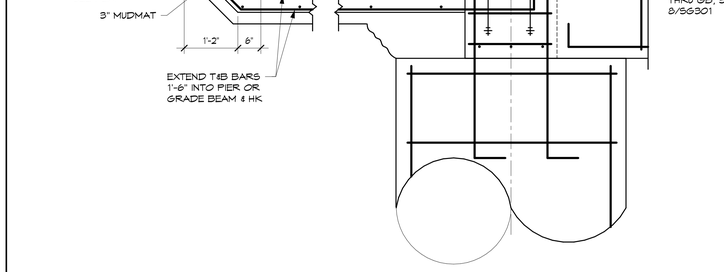

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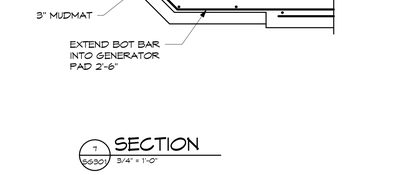

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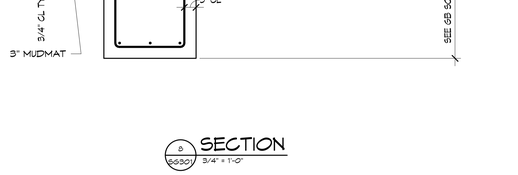

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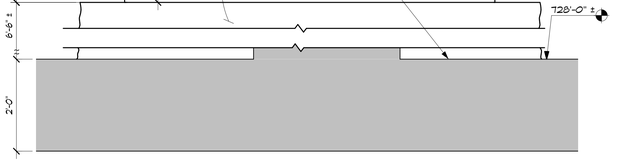

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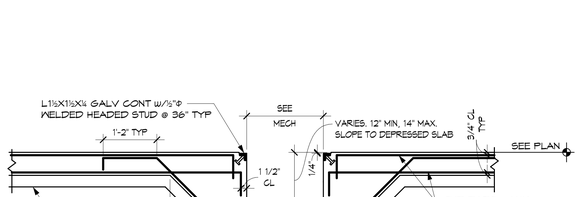

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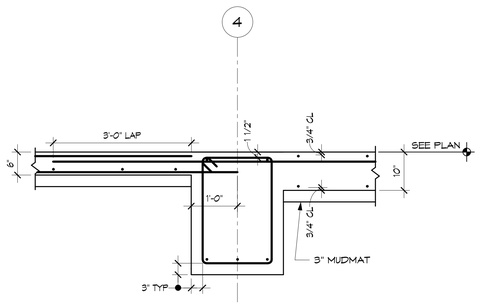

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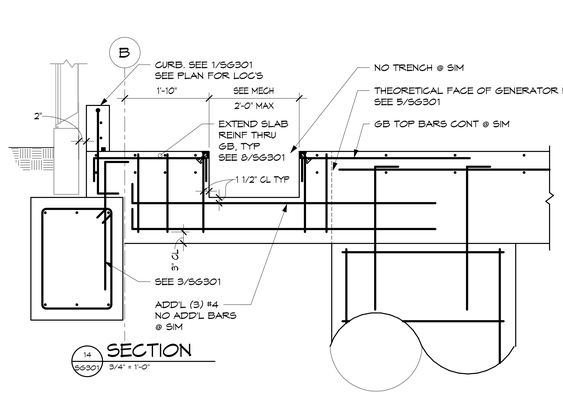

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SECTIONS

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O.S.U. PROJ. NO.	FILENAME	DRAWN BY	DATE	377 X XXXX
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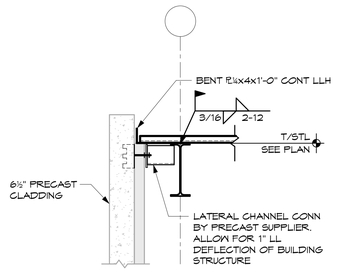

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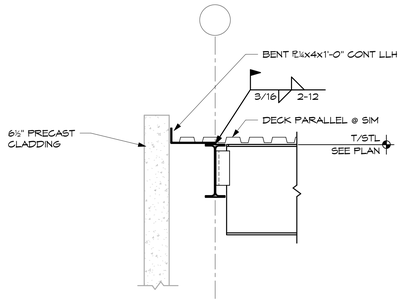

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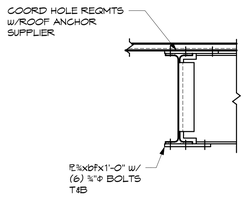

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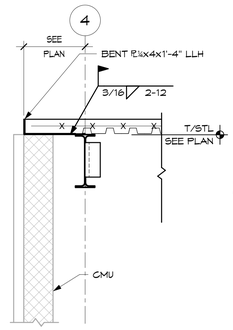

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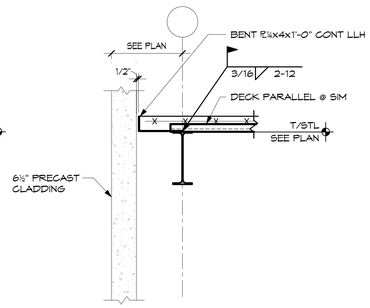

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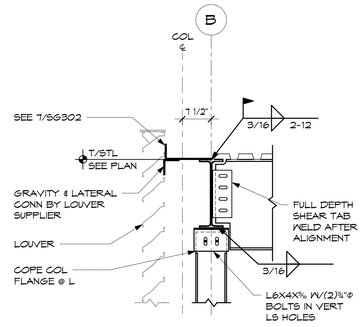

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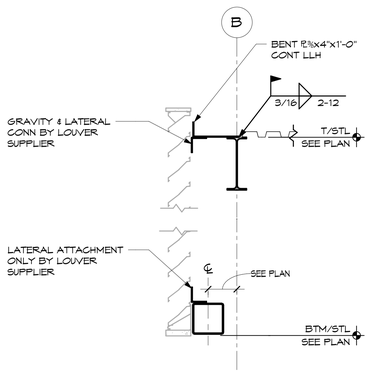

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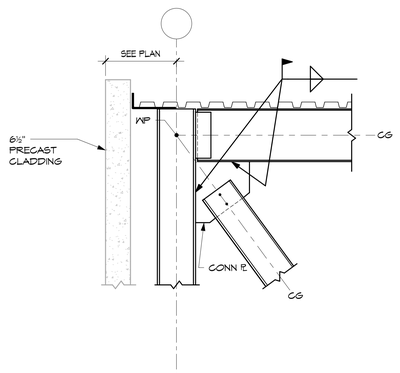

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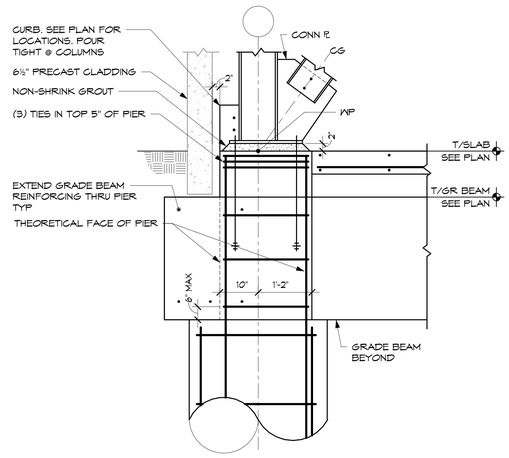
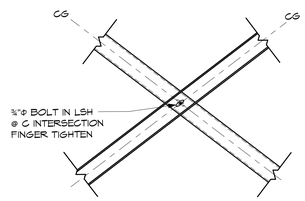
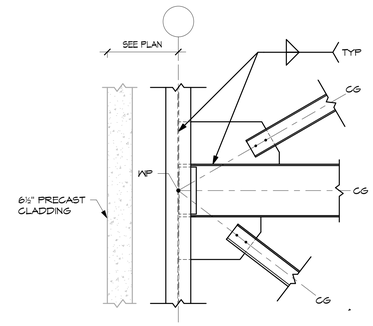


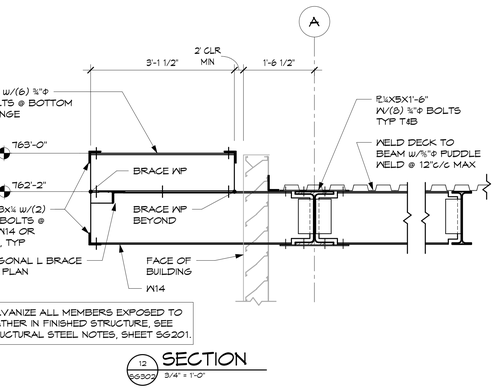
9 DETAIL SG302 3/4" = 1'-0"



10 DETAIL SG302 3/4" = 1'-0"

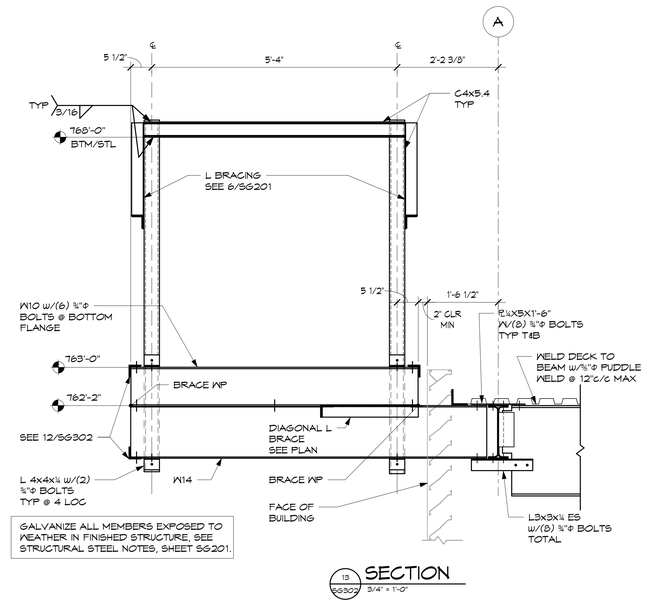


11 DETAIL SG302 3/4" = 1'-0"



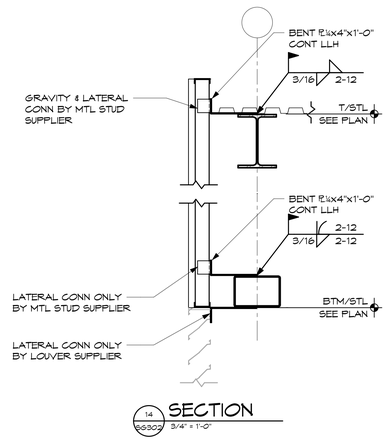
GALVANIZE ALL MEMBERS EXPOSED TO WEATHER IN FINISHED STRUCTURE, SEE STRUCTURAL STEEL NOTES, SHEET SG201.

12 SECTION SG302 3/4" = 1'-0"



GALVANIZE ALL MEMBERS EXPOSED TO WEATHER IN FINISHED STRUCTURE, SEE STRUCTURAL STEEL NOTES, SHEET SG201.

13 SECTION SG302 3/4" = 1'-0"



14 SECTION SG302 3/4" = 1'-0"

DETAIL 10 SG302 3/4" = 1'-0"

ENTIRE SHEET REISSUED

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NOT FOR CONSTRUCTION
- ○ CONCEPTUAL DESIGN
- ○ SCHEMATICS
- ○ DESIGN DEVELOPMENT
- ● CONSTRUCTION DOCUMENTS

THIS DOCUMENT WAS ORIGINALLY ISSUED BY STEPHEN J METZ ON 7-22-10. THIS DOCUMENT IS NOT CONSIDERED A SEALED DOCUMENT.

NO.	DATE	DESCRIPTION
18	4-6-11	Bulletin 018 - Generator Building Revision
9	12-22-10	Bid Package 3 - Addendum 1
8	12-14-10	Bid Documents - Bid Package 3 - Bid Issuance
7	11-30-10	Bid Documents - Bid Package 3 - CD Back Check Set
6	11-17-10	Bid Documents - Bid Package 3 - Revised CD Review Set
5	10-15-10	Bid Documents - Bid Package 3 - CD Review Set
4	09-17-10	Bid Documents – Bid Pack 2 – Back Check
3	07-26-10	ISSUED FOR PERMIT (FOUNDATION ONLY)
2	07-22-10	CD - BID PACK 2
1	05-07-10	DD - ADDENDUM

REVISIONS

OSU PROJECT NUMBER *OSU-090344*

SOUTH CAMPUS CENTRAL CHILLER PLANT

PREPARED FOR
THE OHIO STATE UNIVERSITY
FACILITIES OPERATIONS AND DEVELOPMENT
COLUMBUS, OHIO

PREPARED BY
CHAMPLIN ARCHITECTURE
424 EAST FOURTH STREET *CINCINNATI, OHIO 45202*

SECTIONS

SHEET **SG302**

O.S.U. PROJ. NO.	FILENAME	DRAWN BY	DATE	
OSU-090344	J:\250\A055	SMBH	4-6-11	377 X XXXX

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